

dards EN 196 and EN 197, sulfate-resistant cements are limited in the C_3A content to $< 3\%$ and the Al_2O_3 content to $< 5\%$.

Systematic studies and reviews were done to evaluate the deterioration processes of the secondary-ettringite formation due to an external sulfate attack on hydrated cement paste, mortar and concrete.^{4,6,7} The results showed that the amounts of C_3A (0–8 %) in the cement systems do not necessarily protect them from a sulfate deterioration. In contrast, the w/c ratio of the investigated samples had a major impact on the failure time of the samples during 40 years of exposure under real conditions. The permeability (porosity) of concrete samples has a major influence on the deterioration of the samples. It is generally agreed that the alumino-ferrite phase (C_4AF) seems to be less important with regard to the secondary-ettringite formation during a sulfate attack due to its lower reaction kinetics.^{8,9} The formation of secondary ettringite from C_4AF in the presence of gypsum (sulfate) and the formation of aluminum- (AH_3) and iron-hydroxide (FH_3) are given in Equation (2):



Besides ettringite, gypsum can also form during a sulfate attack, especially in highly concentrated sulfate solutions.^{10,11} The influence of a gypsum formation on the performance of cement pastes, mortars and concretes was studied in detail by many authors.^{6,12–14} It was suggested that the secondary-gypsum formation is related to the amount of alite (C_3S) in the cement system due to the possibility of a portlandite formation. The transformation of portlandite into gypsum can cause an expansion, usually later, during the sulfate exposure, and the softening of the near-surface regions due to the gypsum formation was also observed. The softening was attributed to the decalcification of the C-S-H phase.

At lower temperatures, thaumasite ($CaSiO_3 \cdot CaSO_4 \cdot CaCO_3 \cdot 15H_2O$; C_3SSCH_{15}) is formed in addition to ettringite as a result of the reaction between C-S-H and SO_4^{2-} , CO_2 or CO_3^{2-} , and water. The AFt phase is structurally similar to ettringite, with Al^{3+} substituted by Si^{4+} . Thaumasite is more stable at lower temperatures since silicon tends to adopt the octahedral co-ordination. However, thaumasite is formed also at temperatures around 20 °C and above.^{15,16} Once thaumasite is formed, it remains stable up to 30 °C.¹⁷ The formation of thaumasite always needs a source of carbonate which can be supplied from the limestone contained in the cement itself. The damage due to the thaumasite formation was investigated by various authors.^{18–21}

Thus, the interactions of sulfate ions with the cement matrix result in a disruption of the concrete and a significant loss of the mechanical strength and mass, and it leads to a reduction of the service life of concrete composites.

This paper is focused on the sulfate attack on fine-grained concrete, investigating the effect of 0.5 % sulfuric acid (simulating *MICC*) on the microstructural

changes of various types of concrete after a treatment for the period of six months. The changes in the microstructure were determined with mercury intrusion porosimetry and scanning electron microscope. The aim of this study was to compare the resistances of various types of concrete against the sulfate attack, in view of the microstructural changes that may result in a disintegration and the subsequent loss of the concrete material.

2 MATERIALS AND METHODS

Fine-grained concrete specimens (40 mm × 40 mm × 160 mm) were prepared with a water-to-binder ratio of 0.45 and three fractions of quartz sand according to Czech standard CSN 72 1200, with designations PG 1 (< 0.5 mm), PG 2 (0.5–1 mm) and PG 3 (1–2.5 mm), in the weight-to-binder ratio of 1 : 1 : 1 : 1. Seven concrete mixtures with different compositions of the binder were prepared: PC – Portland cement CEM I 42.5 R (100 %); SRP – sulfate-resisting Portland cement (100 %); SRS – sulfate-resisting slag cement CEM III/B 32.5 N-SV (100 %); MK – metakaolin (20 %), Portland cement (80 %); GL – ground limestone (20 %), Portland cement (80 %); GBFS – granulated blast-furnace slag (20 %), Portland cement (80 %) and FA – low-calcium fly ash (20 %), Portland cement (80 %). The mixtures with the supplementary cementing materials (metakaolin, limestone, slag and fly ash) corresponded to Portland blended cements according to European standard EN 197-1. The chemical compositions and physical properties of the initial materials are given in **Table 1**. The specimens were unmolded 24 h after the casting under laboratory conditions ($t = (22 \pm 2)$ °C, R. H. was (55 ± 5) %) and

Table 1: Chemical compositions and physical properties of initial materials in mass fractions, w/%

Tabela 1: Kemijska sestava in fizikalne lastnosti izhodnih materialov v masnih deležih, w/%

Materials	PC	SRP	SRS	MK	GL	GBFS	FA	
Chemical composition	CaO	61.48	60.92	47.53	0.20	54.73	34.81	3.60
	SiO ₂	21.26	21.88	32.84	58.70	0.81	39.78	53.70
	Al ₂ O ₃	5.08	3.65	6.01	38.50	0.32	8.17	24.62
	Fe ₂ O ₃	3.64	4.36	1.54	0.72	0.10	1.54	7.91
	SO ₃	2.42	2.39	2.30	0.02	0.05	0.46	0.96
	MgO	0.86	3.15	7.21	0.38	0.38	13.22	1.67
	Na ₂ O	0.12	0.41	0.32	0.04	–	–	0.85
	K ₂ O	0.91	0.86	0.65	0.85	–	–	2.62
	MnO	0.07	0.11	0.03	–	0.01	0.89	–
	TiO ₂	0.29	0.34	0.41	0.49	–	0.24	1.03
	Cr ₂ O ₃	–	–	–	–	–	0.14	–
	P ₂ O ₅	–	–	–	–	–	0.03	2.25
	L.O.I.	4.17	1.97	0.82	1.67	43.99	1.48	2.82
Physical properties	SSA (m ² kg ⁻¹)	360	685	504	13060	390	384	340
	SG (kg m ⁻³)	3120	2650	2950	1070	2700	2810	2300

Note: "–" ... not tested

placed into a water bath for another 27 d. Afterwards, the specimens were air-dried for 24 h and then the pore structures of the samples were studied to determine the total porosity and the pore-size distribution with high-pressure mercury intrusion porosimetry using a Micromeritics PoreSizer 9310 porosimeter, and the microstructures of samples were observed with a scanning electron microscope, MIRA3 (TESCAN), equipped with an EDX probe.

Subsequently, the test samples were covered with a protective coat to prevent drying and they were placed into a solution of 0.5 % H_2SO_4 for a period of six months. The concentration of the sulfuric acid was chosen in accordance with the literature.²² The solution level was maintained at a height of 5 mm and the solutions were weekly renewed. After six months, the samples were slit lengthwise and the changes in their microstructures were studied with high-pressure mercury intrusion porosimetry and scanning electron microscope. Attention was paid mainly to the lower parts of the samples, near the H_2SO_4 solution.



Figure 1: Lower part of SRP concrete sample after being in H_2SO_4 0.5 % for 6 months

Slika 1: Spodnji del SRP-betonskega vzorca po namakanju 6 mesecev v H_2SO_4 0,5 %

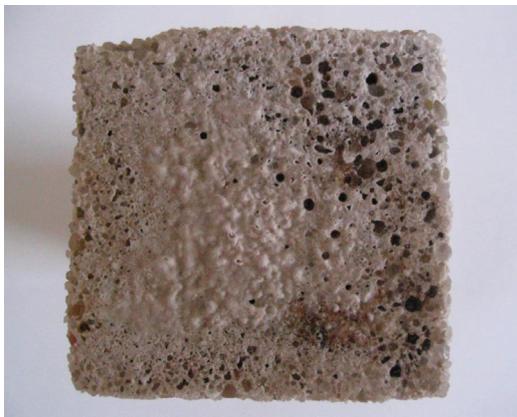


Figure 2: Immersed part of SRP concrete sample after being in H_2SO_4 0.5 % for 6 months

Slika 2: Potopljeni del SRP-betonskega vzorca po namakanju 6 mesecev v H_2SO_4 0,5 %

3 RESULTS AND DISCUSSION

3.1 Visual appearance

After being in 0.5 % H_2SO_4 for six months, a visible degradation of the submerged part occurred on all the investigated samples and the creation of a rust layer on the periphery of each sample, just above the level of the acid, took place. **Figure 1** illustrates the effect of the 0.5 % sulfuric acid on the SRP-concrete sample. A detailed view of the immersed part of the sample is shown in **Figure 2**. There are well recognizable exposed aggregates and white precipitates of gypsum ($CaSO_4 \cdot 2H_2O$) on the surface.

3.2 Changes in the porosity

It was found that all the concrete samples had almost identical pore distributions before the sulfate attack. The samples mainly contained pores with a diameter of 0.1 μm (**Figure 3**). The largest porosity was observed for the FA concrete and the lowest for the SRS concrete. The results of the determination of the cumulative pore volumes of the samples exposed to the sulfate attack for six months are also in **Figure 3**. The pore-size distribution remained approximately the same, but the samples had a slightly higher amount of the pores with a diameter below 0.1 μm . This means that larger pores were filled in to a certain degree by the products of the reactions of the

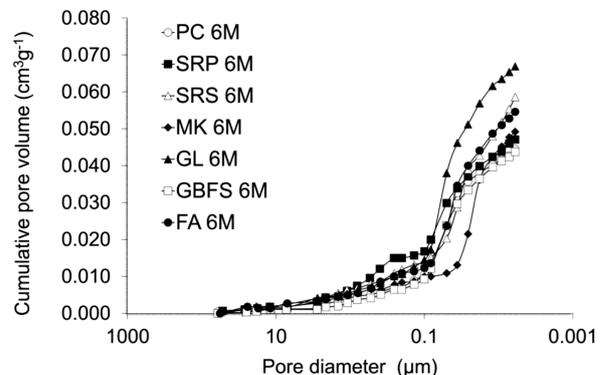
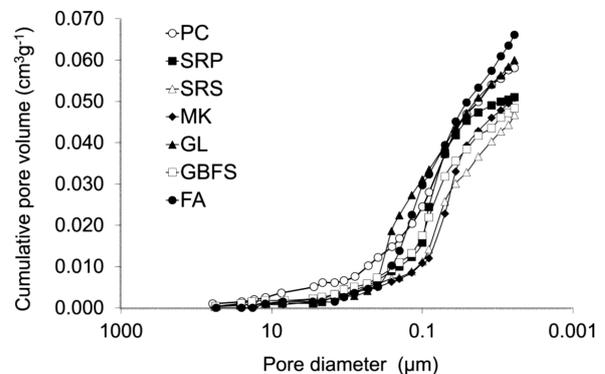


Figure 3: Pore-size distribution of the investigated concrete samples before and after a 6-month (6M) exposure to H_2SO_4 0.5 %

Slika 3: Razporeditev velikosti por v preiskovanih vzorcih betona pred 6-mesečno (6M) izpostavitvijo H_2SO_4 0,5 % in po njej

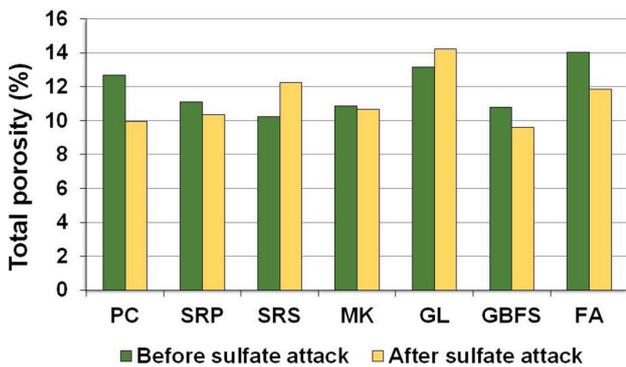


Figure 4: Total porosity of the concrete samples before and after a 6-month exposure to 0.5 % H₂SO₄

Slika 4: Skupna poroznost betonskih vzorcev pred 6-mesečno izpostavitvijo H₂SO₄ 0,5 % in po njej

sulfate attack. The total porosities of the investigated concrete samples are depicted in **Figure 4**. The reduction in the total porosity of most samples occurred after the sulfate attack, which indicates that the pores were filled in by the products of the sulfate corrosion of the concrete.

The most significant decrease was observed for the PC and FA concretes. Conversely, for the SRS and GL concretes, an increase in the total porosity was found. In the case of the SRS concrete, it is apparently due to the low porosity before the sulfate attack, prohibiting a smooth crystallization of the sulfate-corrosion products in the pores and a disruption of the microstructures of the samples. The increase in the total porosity of the GL concrete is in accordance with the conclusions of E. F.

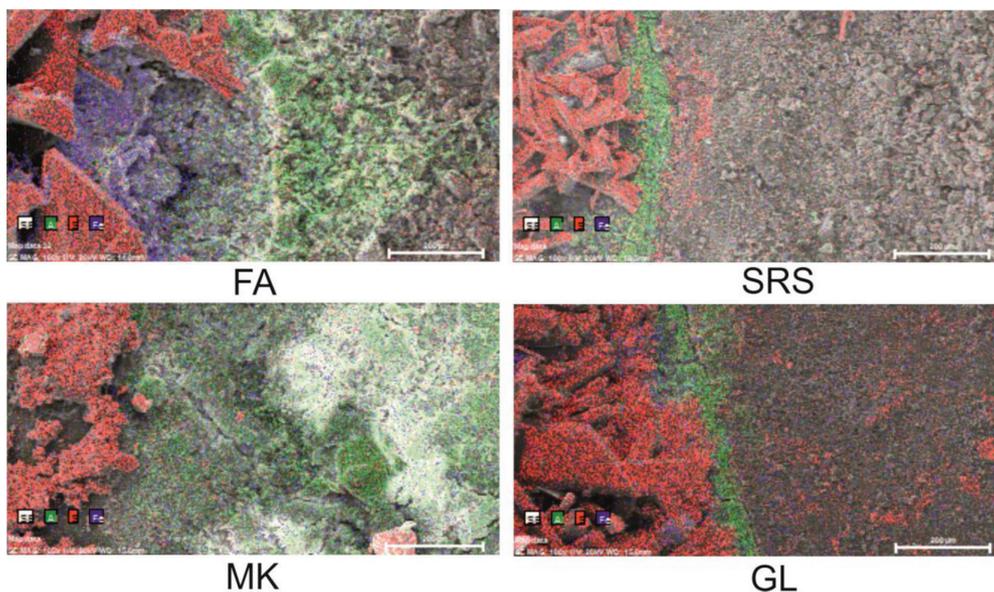


Figure 5: Distribution of sulfur (red), aluminum (green) and iron (blue) on the surfaces of lower parts of lengthwise slices of the selected types of concrete samples obtained with EDX SEM

Slika 5: EDX-SEM-razporeditev žvepla (rdeče), aluminija (zeleno) in železa (modro) na površini spodnjega dela vzdolžnega prereza izbranih vrst betona

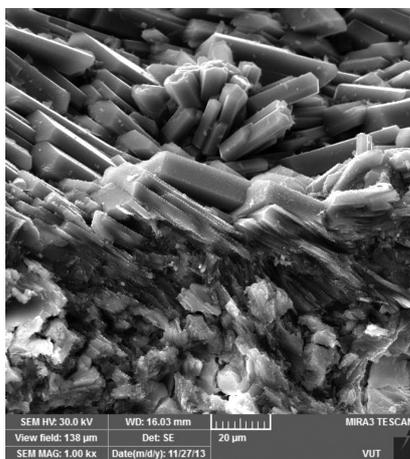


Figure 6: SEM image of gypsum identified in the structure of submerged part of PC-concrete sample after a 6-month sulfate attack

Slika 6: SEM-posnetek sadre, dobljene v potopljenem delu PC-betonskega vzorca po 6-mesečni izpostavitvi sulfatu

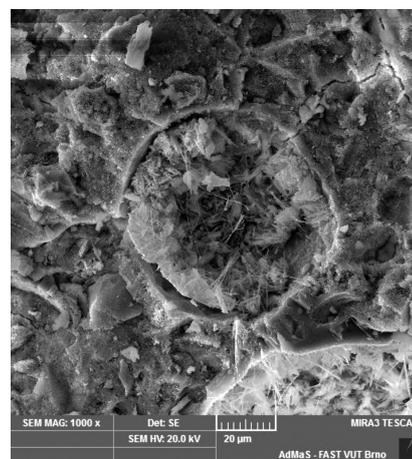


Figure 7: SEM image of microstructure of PC-concrete sample after a 6-month sulfate attack

Slika 7: SEM-posnetek mikrostrukture vzorca PC-betona po 6-mesečni izpostavitvi sulfatu

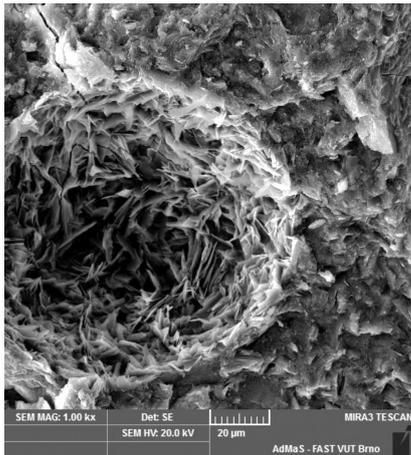


Figure 8: SEM image of microstructure of MK-concrete sample after a 6-month sulfate attack

Slika 8: SEM-posnetek mikrostrukture vzorca MK-betona po 6-mesečni izpostavitvi sulfatu

Irassar et al.¹² He found that a concrete containing a limestone filler is more susceptible to a sulfate attack and less durable than the corresponding plain mortar, as indicated by its larger expansion, greater surface deterioration, deeper transition zone of the attack, larger deposition of gypsum and higher degree of the CH depletion.

3.3 Changes in the microstructure

By mapping the chemical-element distribution on the surface of the lower part of the lengthwise slice of each samples, the presence of the locations rich in sulfur, iron and aluminum was determined. In the lowest parts of the samples, sulfur prevailed in the gypsum form. Using EDX REM, an aluminum-rich zone, identified as hydrated $\text{Al}(\text{OH})_3$, was located just above the gypsum. The presence of $\text{Al}(\text{OH})_3$ is presumably caused by the formation of ettringite from C_4AF in the presence of gypsum

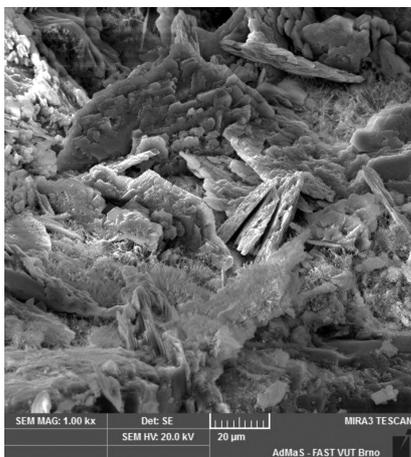


Figure 9: SEM image of microstructure of GL-concrete sample after a 6-month sulfate attack

Slika 9: SEM-posnetek mikrostrukture vzorca GL-betona po 6-mesečni izpostavitvi sulfatu

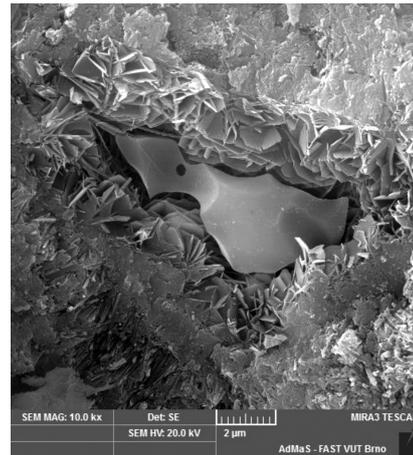


Figure 10: SEM image of microstructure of GBFS-concrete sample after a 6-month sulfate attack – detail of a partially reacted slag grain

Slika 10: SEM-posnetek mikrostrukture vzorca GBFS-betona po 6-mesečni izpostavitvi sulfatu – detajl delno zreagirane zrna žilindre

(Equation (2)). Thus, it can be concluded that even during a six-month sulfate attack, ettringite forms from C_4AF . In the case of the samples made from the materials with a high iron content (FA, PC, SRP), the zone rich in iron was present between the sulfur and aluminum locations. The iron compound was not sufficiently established; it is just known that it is a silicate without aluminum. The identification of this compound is still in process. Selected EDX REM images of the mapped chemical-element distributions are shown in **Figure 5**. The PC-, SRP- and FA-concrete samples had very similar distributions of chemical elements, and so did SRS and GBFS. The MK sample did not have a narrow zone rich in aluminum because of a high aluminum content in the used metakaolin.

In the submerged parts of all the investigated samples, typical prismatic gypsum crystals were identified with EDX SEM (**Figure 6**). The pores of the PC-concrete sample were largely filled with the crystalline

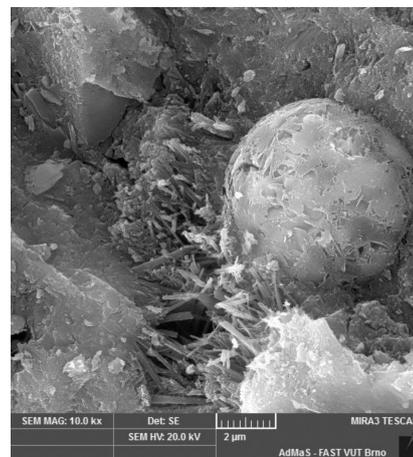


Figure 11: SEM image of microstructure of FA-concrete sample after a 6-month sulfate attack

Slika 11: SEM-posnetek mikrostrukture vzorca FA-betona po 6-mesečni izpostavitvi sulfatu

products of the sulfate attack, particularly with the typical needle crystals of ettringite (**Figure 7**), confirming the reduction in the total porosity. Of all the investigated concrete samples, the SRP- and SRS-concrete samples showed the smallest changes in the microstructures as just small amounts of ettringite were identified.

In the microstructures of the MK- and GL-concrete samples, a lot of portlandite in typical hexagonal crystals was identified on the surfaces of the pores (**Figures 8 and 9**), in contrast to the microstructures of these samples before the sulfate attack. An increased amount of portlandite in the pores was observed in all the studied samples, but for the MK and GL samples, this amount was enormous. This increased presence of portlandite was probably caused by the capillary action of the solution while the samples were kept in H₂SO₄. A direct contact of the cement matrix with the solution of sulfuric acid leads to a sulfate attack and the formation of gypsum. The non-aggressive solution penetrates to the higher parts of a sample, dissolves the surrounding Ca(OH)₂ and, during the drying of the sample, its crystallization occurs in the pores. For this reason, in the lowest parts of the samples, only gypsum as the primary product of the sulfate attack was identified. In addition to gypsum, the higher parts of the samples also contained ettringite and other by-products of the sulfate attack. In the case of the GBFS-concrete sample, crystals of portlandite were concentrated in the crevices around the partially reacted slag grains (**Figure 10**). Ettringite was identified on the surfaces of the pores. The FA-concrete sample contained a lot of damaged particles of fly ash and needle crystals of ettringite (**Figure 11**).

4 CONCLUSIONS

Microstructural changes in various types of fine-grained concrete after a six-month sulfate attack were investigated with mercury intrusion porosimetry and scanning electron microscopy. It was found that the total porosity of most samples was decreased after the sulfate attack, indicated by the products of the sulfate corrosion filling in the pores of the concrete. The most significant decrease was observed for the PC and FA concretes. The smallest changes in the microstructure occurred in the samples made from the sulfate-resisting cements. Thereby, the suitability of their use in the case of a sulfate attack was confirmed. The formation of the locations rich in sulfur, iron and aluminum during the sulfate attack on the concrete was determined by mapping the chemical-element distribution. Due to the presence of Al(OH)₃, which was identified in the aluminum-rich location, it can be concluded that, even during a six-month sulfate attack, ettringite is formed from C₄AF. The fate of the iron originating from C₄AF is still not clear.

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5 REFERENCES

- ¹ A. Neville, *Cem. Concr. Res.*, 24 (2004), 1275–1296, doi:10.1016/j.cemconres.2004.04.004
- ² H. Yuan, P. Dangla, P. Chatellier, T. Chaussadent, *Cem. Concr. Res.*, 53 (2013), 267–277, doi:10.1016/j.cemconres.2013.08.002
- ³ D. Stein, *Instandhaltung von Kanalisationen*, 3rd ed., Ernst, Berlin 1999, 141
- ⁴ J. Skalný, J. Marchand, I. Odler, *Sulfate attack on concrete*, 1st ed., Spon Press, London 2002
- ⁵ H. F. W. Taylor, R. S. Gollop, *Mechanisms of chemical degradation of cement-based systems*, 1st ed., E & FN Spon, London 1997, 177–184
- ⁶ P. J. Monteiro, K. E. Kurtis, *Cem. Concr. Res.*, 33 (2003), 987–993, doi:10.1016/S0008-8846(02)01097-9
- ⁷ R. P. Khatari, V. Sirivivatnanon, *Cem. Concr. Com.*, 27 (1997), 1179–1189, doi:10.1016/S0008-8846(97)00119-1
- ⁸ H. F. W. Taylor, *Cement Chemistry*, 1st ed., Thomas Telford, London 1997, doi:10.1680/cc25929
- ⁹ B. Lothenbach, E. Wieland, *Waste Management*, 26 (2006), 706–719, doi:10.1016/j.wasman.2006.01.023
- ¹⁰ R. Gollop, H. F. W. Taylor, *Cem. Concr. Res.*, 26 (1996), 1013–1028, doi:10.1016/0008-8846(96)00089-0
- ¹¹ R. Gollop, H. F. W. Taylor, *Cem. Concr. Res.*, 22 (1992), 1027–1038, doi:10.1016/0008-8846(92)90033-R
- ¹² E. F. Irassar, V. L. Bonavetti, M. Gonzalez, *Cem. Concr. Res.*, 33 (2003), 31–41, doi:10.1016/S0008-8846(02)00914-6
- ¹³ J. Marchand, E. Samson, Y. Maltais, J. J. Beaudoin, *Cem. Concr. Com.*, 24 (2002), 317–329, doi:10.1016/S0958-9465(01)00083-X
- ¹⁴ M. Santhanam, M. D. Cohen, J. Olek, *Cem. Concr. Res.*, 31 (2001), 845–851, doi:10.1016/S0008-8846(01)00510-5
- ¹⁵ S. Diamond, *Cem. Concr. Com.*, 25 (2003), 1161–1164, doi:10.1016/S0958-9465(03)00138-0
- ¹⁶ M. Collepardi, *Cem. Concr. Com.*, 21 (1999), 147–154, doi:10.1016/S0958-9465(98)00044-4
- ¹⁷ D. Macphee, S. J. Barnett, *Cem. Concr. Res.*, 34 (2004), 1591–1598, doi:10.1016/j.cemconres.2004.02.022
- ¹⁸ N. J. Crammond, *Cem. Concr. Res.*, 15 (1985), 1039–1050, doi:10.1016/0008-8846(85)90095-X
- ¹⁹ G. R. Gouda, D. M. Roy, A. Sarkar, *Cem. Concr. Res.*, 5 (1975), 519–522, doi:10.1016/0008-8846(75)90026-5
- ²⁰ M. Romer, *Cem. Concr. Com.*, 25 (2003), 1173–1176, doi:10.1016/S0958-9465(03)00155-0
- ²¹ M. Romer, L. Holzer, M. Pfiffner, *Cem. Concr. Com.*, 25 (2003), 1111–1117, doi:10.1016/S0958-9465(03)00141-0
- ²² N. De Belie, J. Montanya, A. Beeldens, E. Vinckec, D. Van Gemert, W. Verstraetec, *Cem. Concr. Res.*, 34 (2004), 2223–2236, doi:10.1016/j.cemconres.2004.02.015